

MARSEILLE
DU 27 MAI
AU 3 JUIN
2022



ICOLD
27TH CONGRESS
90TH ANNUAL
MEETING



CIGB
27^{ÈME} CONGRÈS
90^{ÈME} RÉUNION
ANNUELLE



Special Sessions

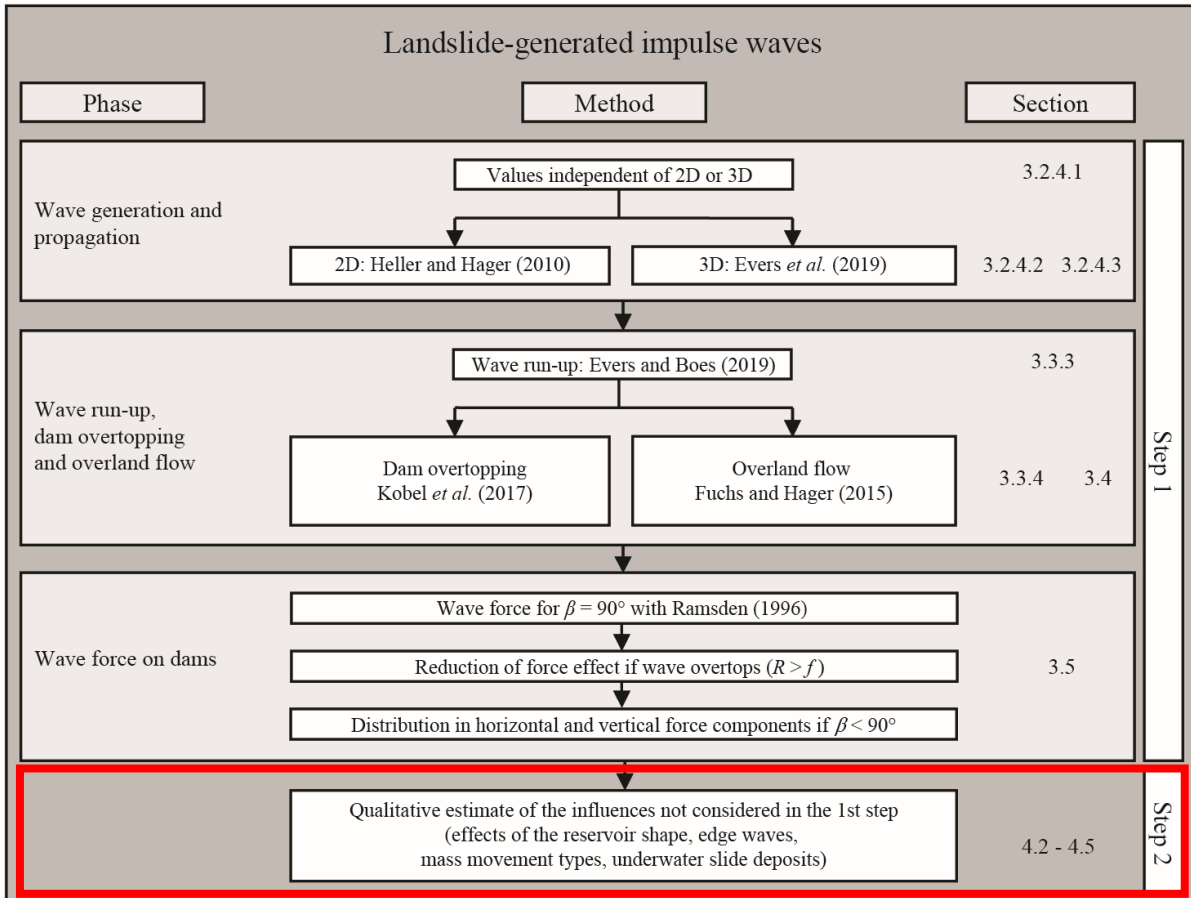
Workshop «LANDSLIDE-GENERATED IMPULSE WAVES IN RESERVOIRS»

Manual Step 2: Water body geometry, Edge waves, Sensitivity

Frederic Evers
ETH Zurich



Computational procedure – Overview

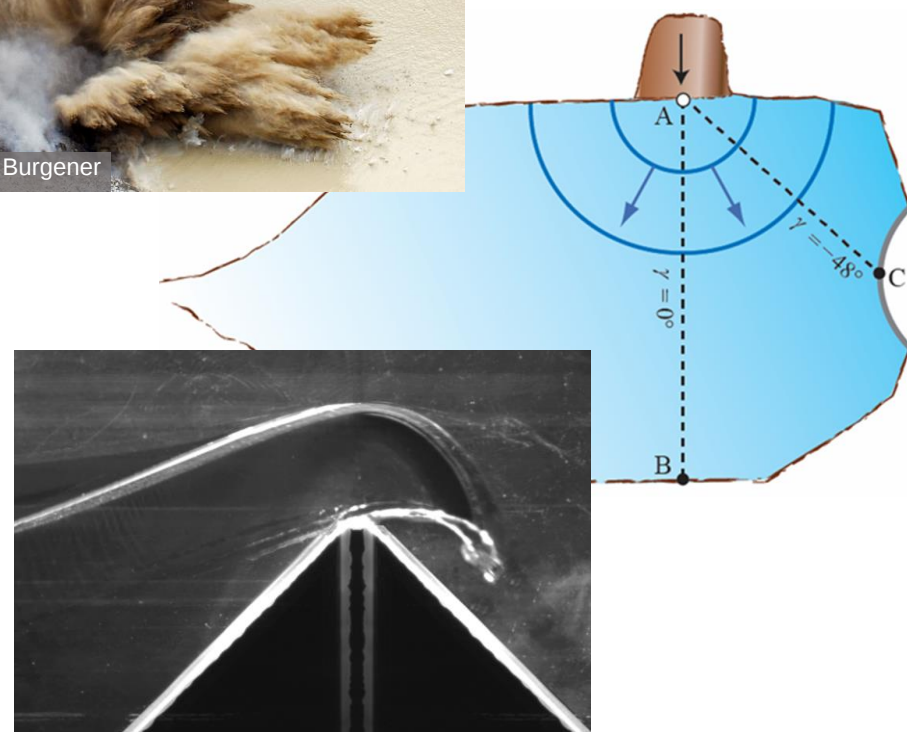


→ «Step 2»

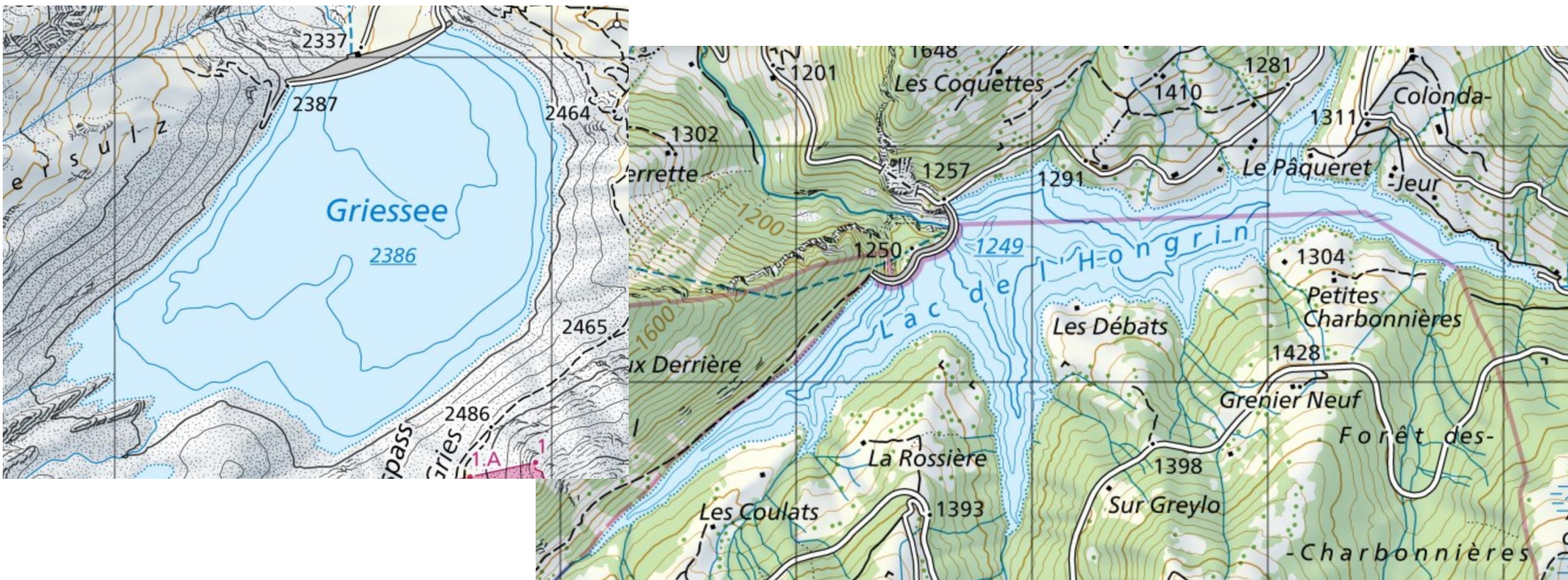


«Step 2» – Outline

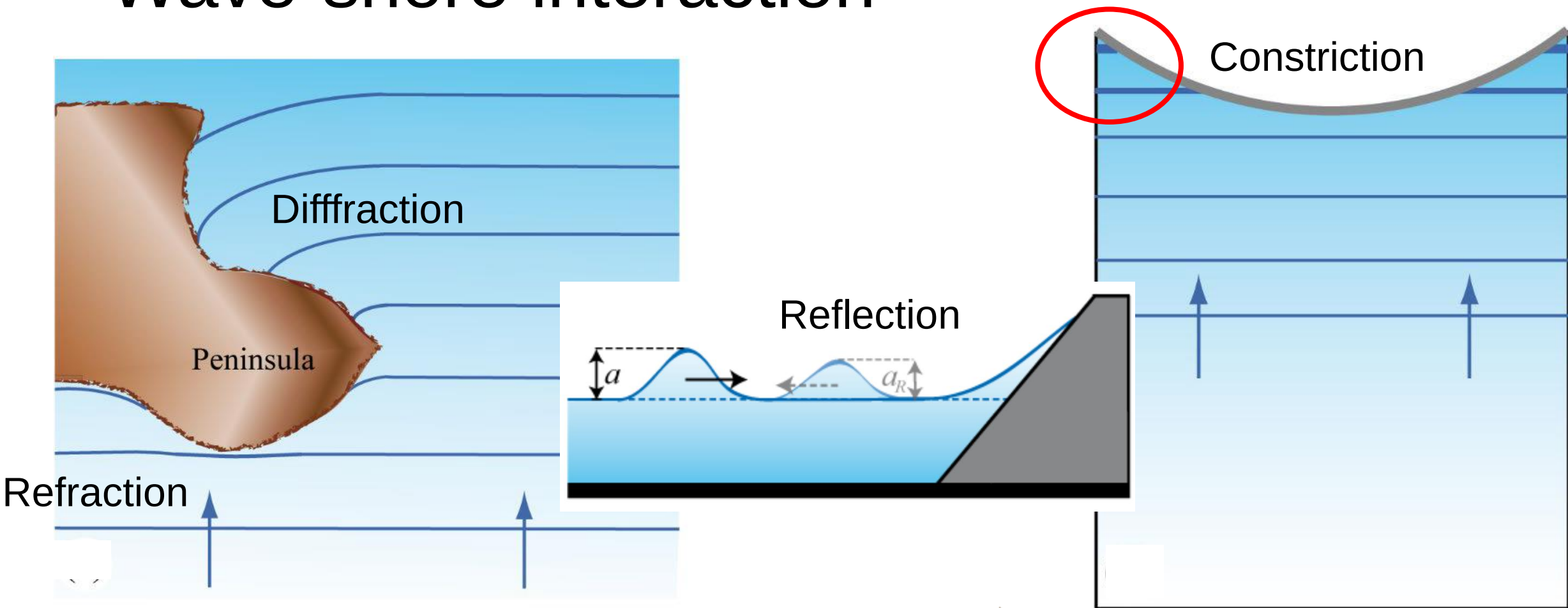
- Reservoir shape
- Edge waves
- Mass movement types
- Sensitivity and uncertainty



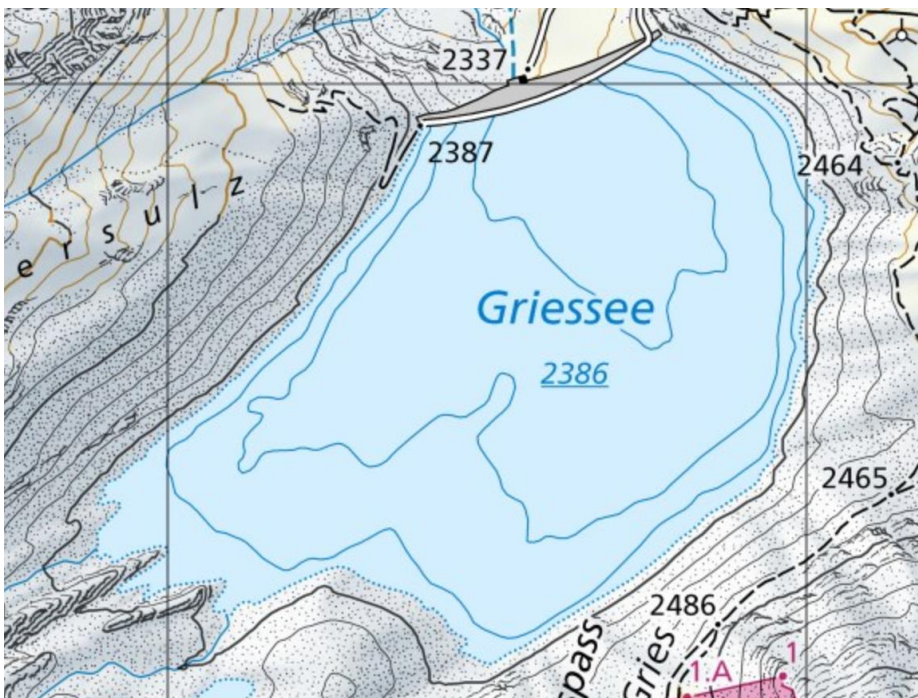
Reservoir shape



Wave-shore interaction

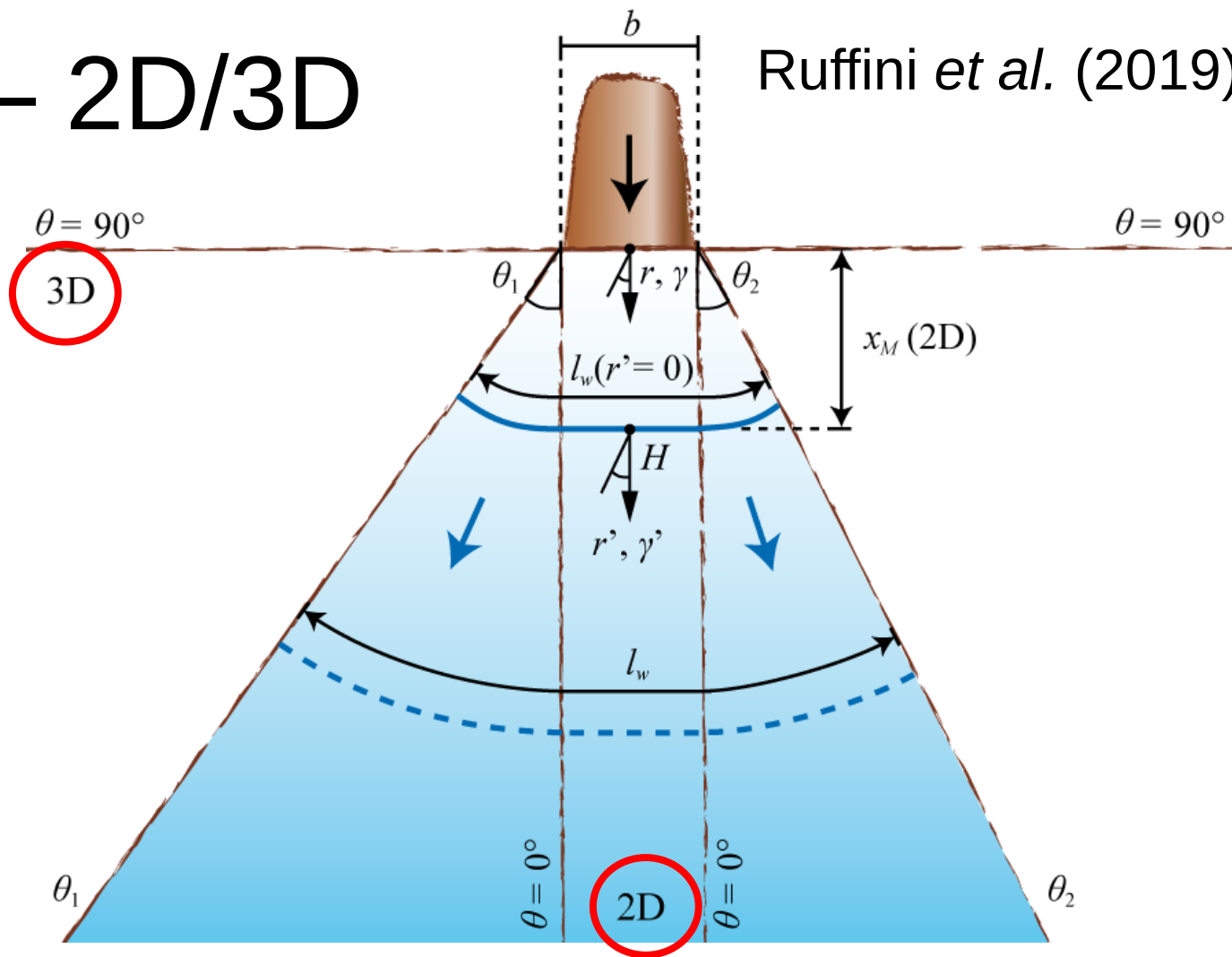
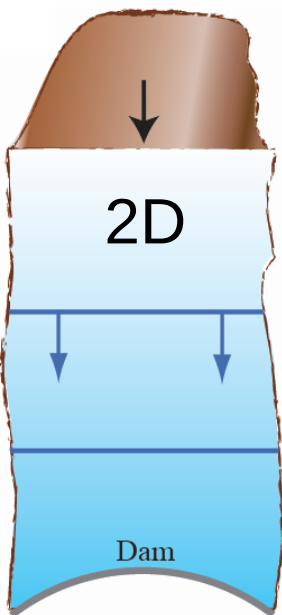
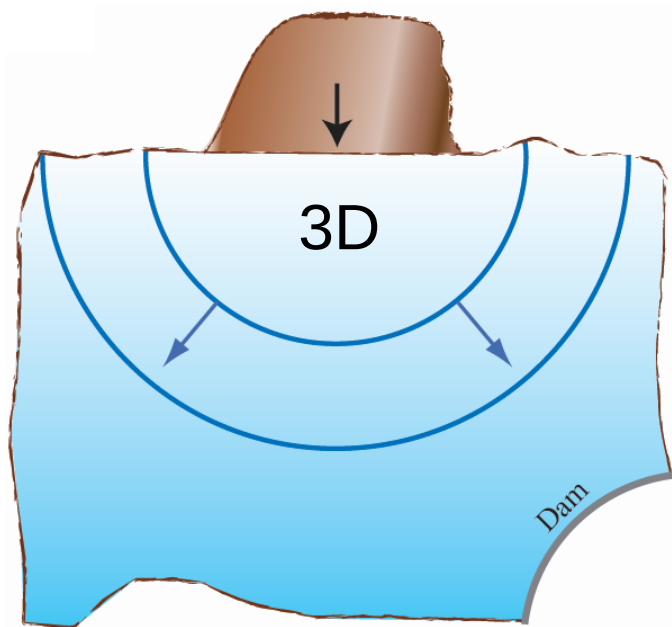


Reservoir shape



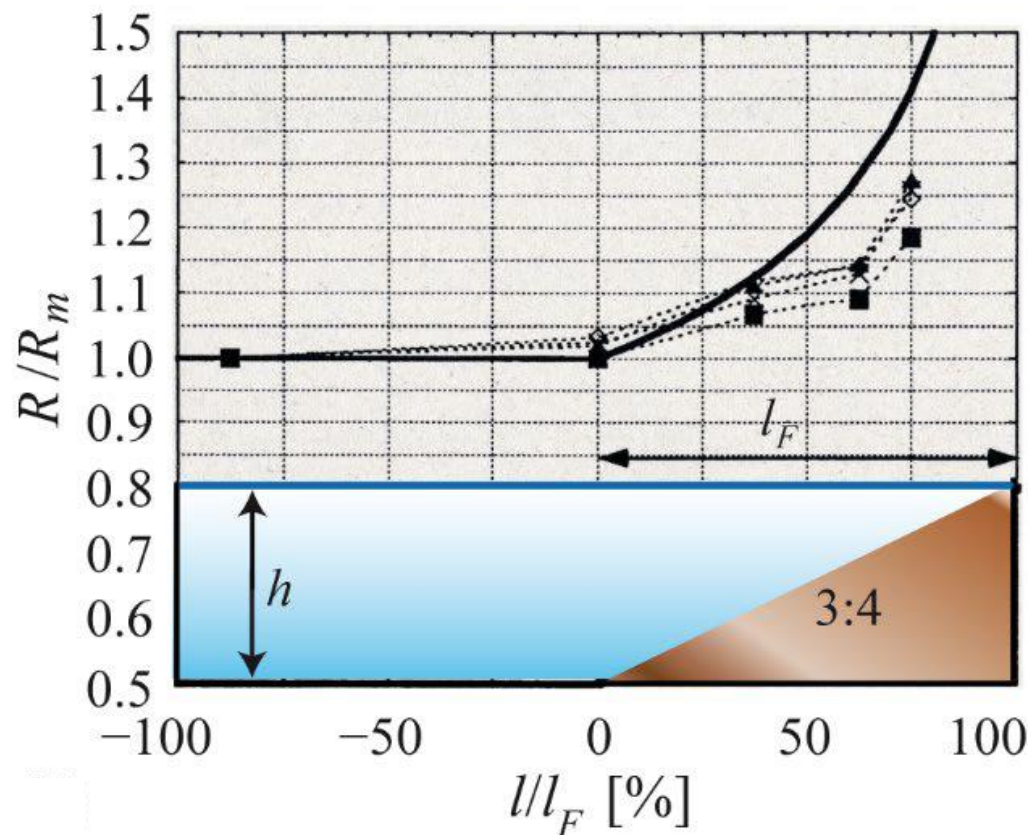
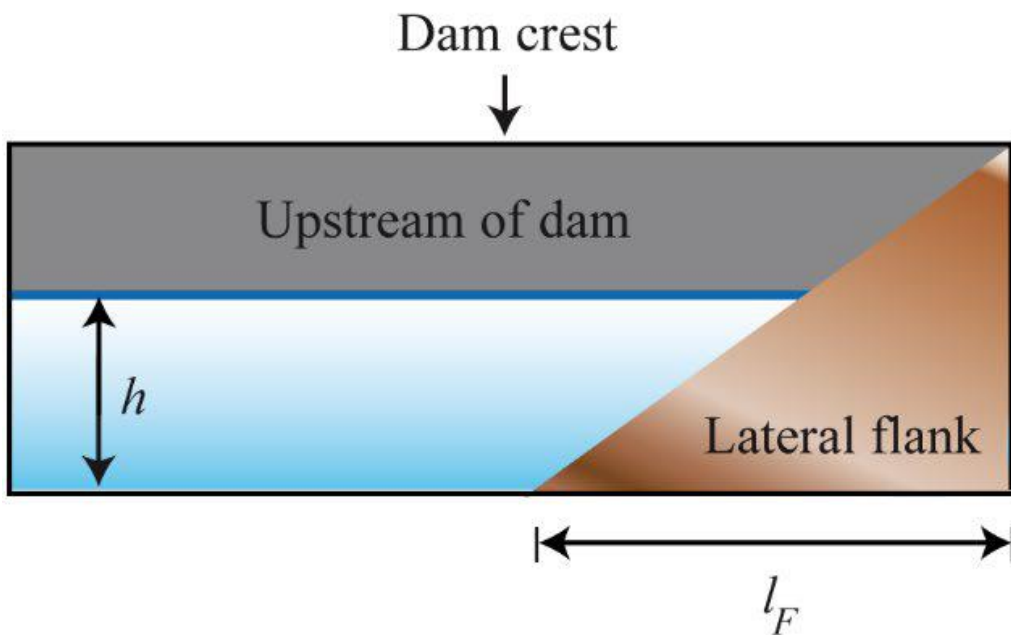
Reservoir shape – 2D/3D

Ruffini *et al.* (2019)

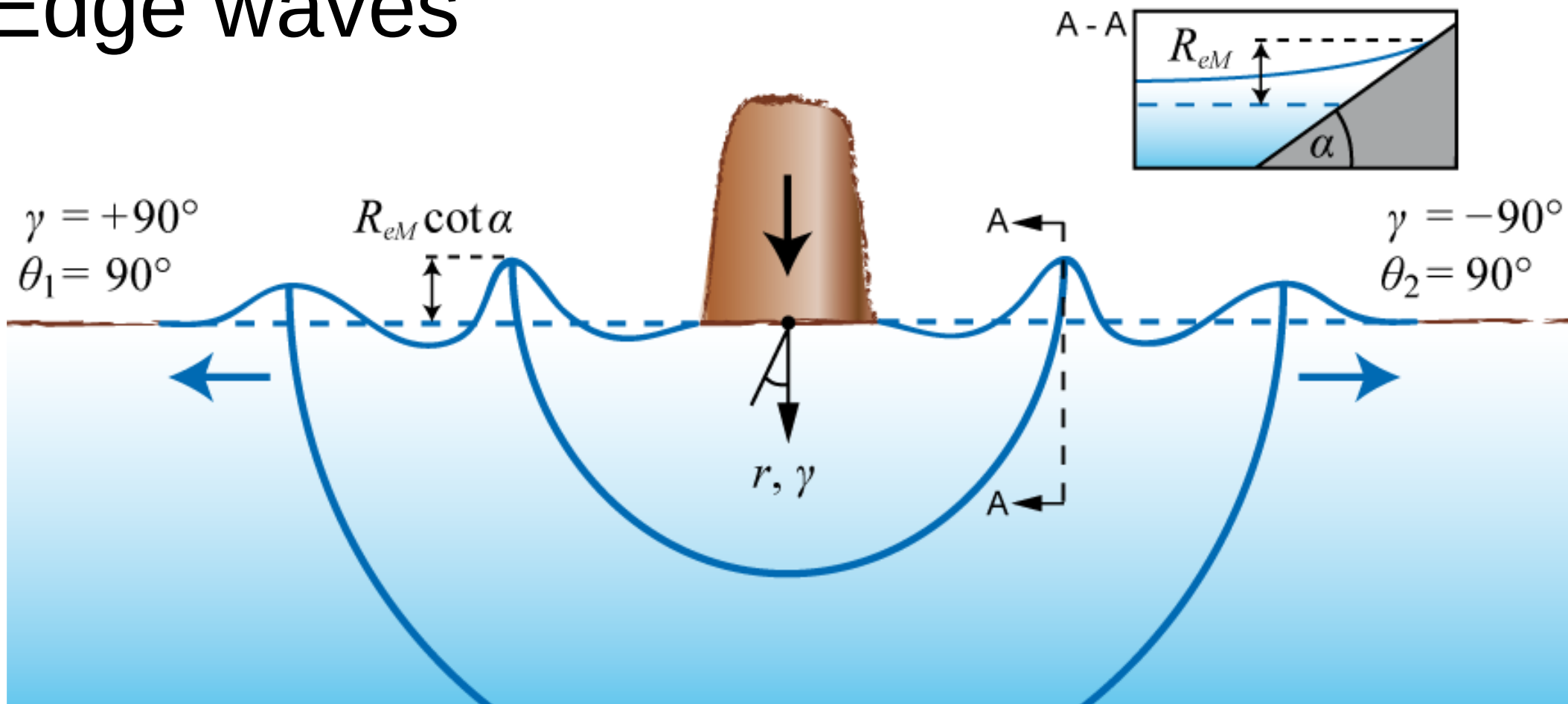


Reservoir shape – Lateral flanks

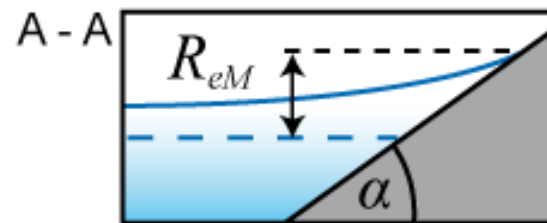
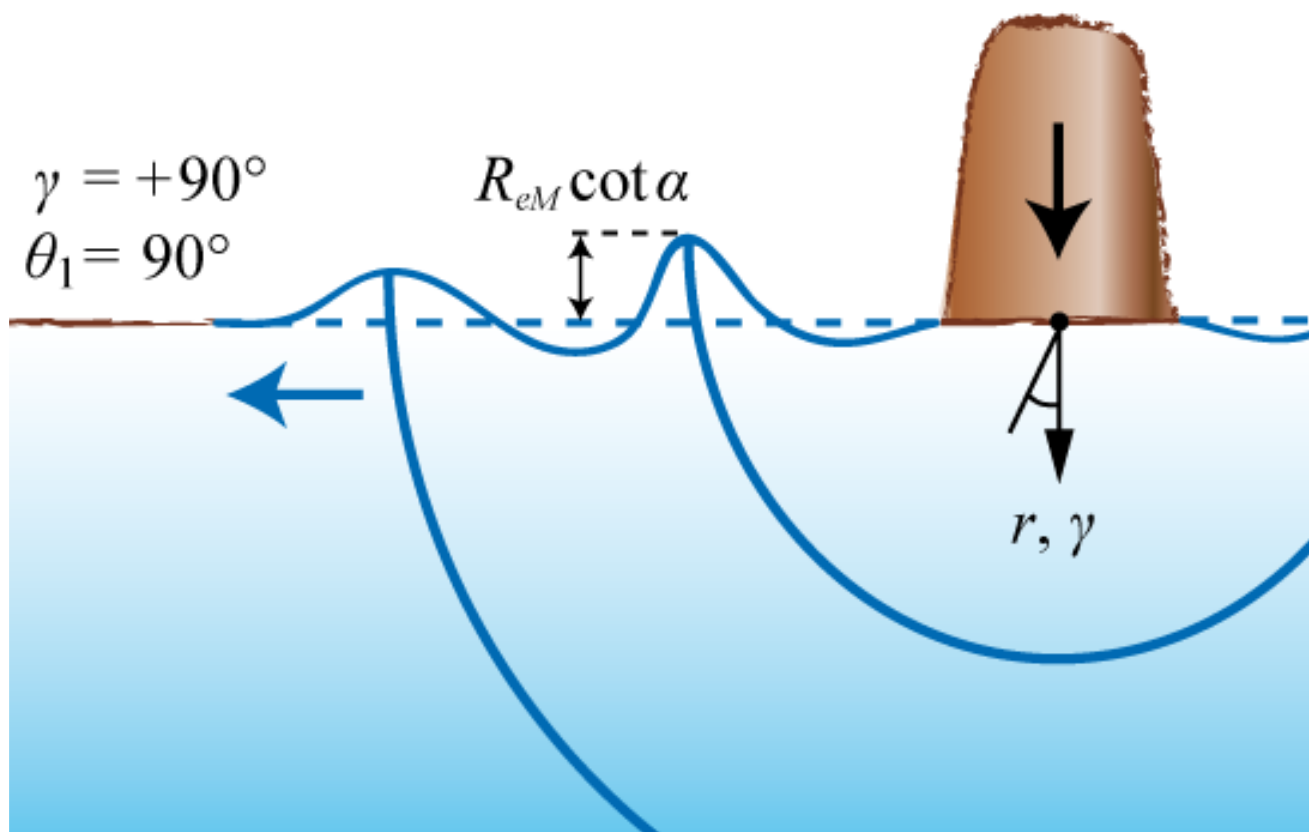
Müller (1995)



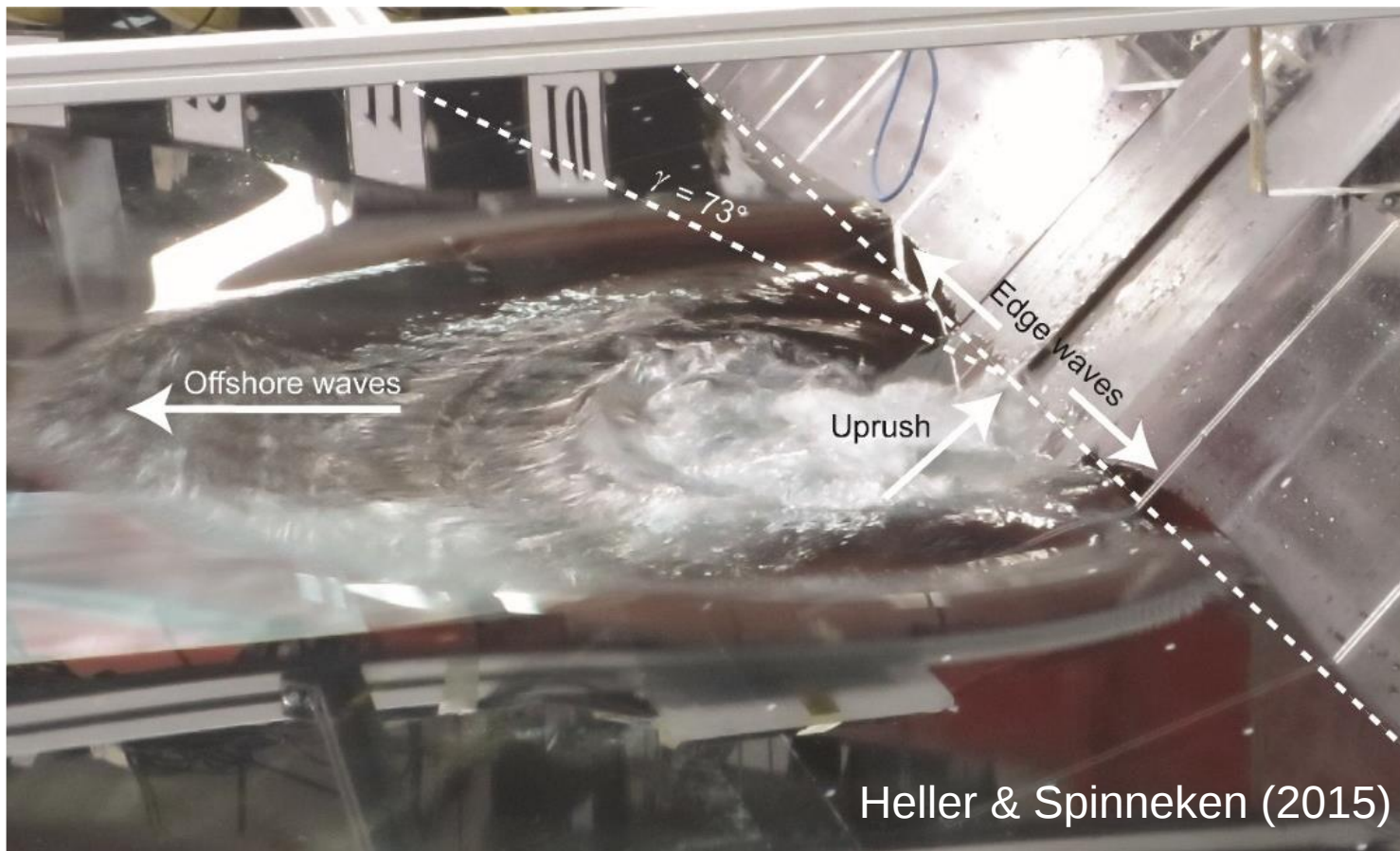
Edge waves



Edge waves



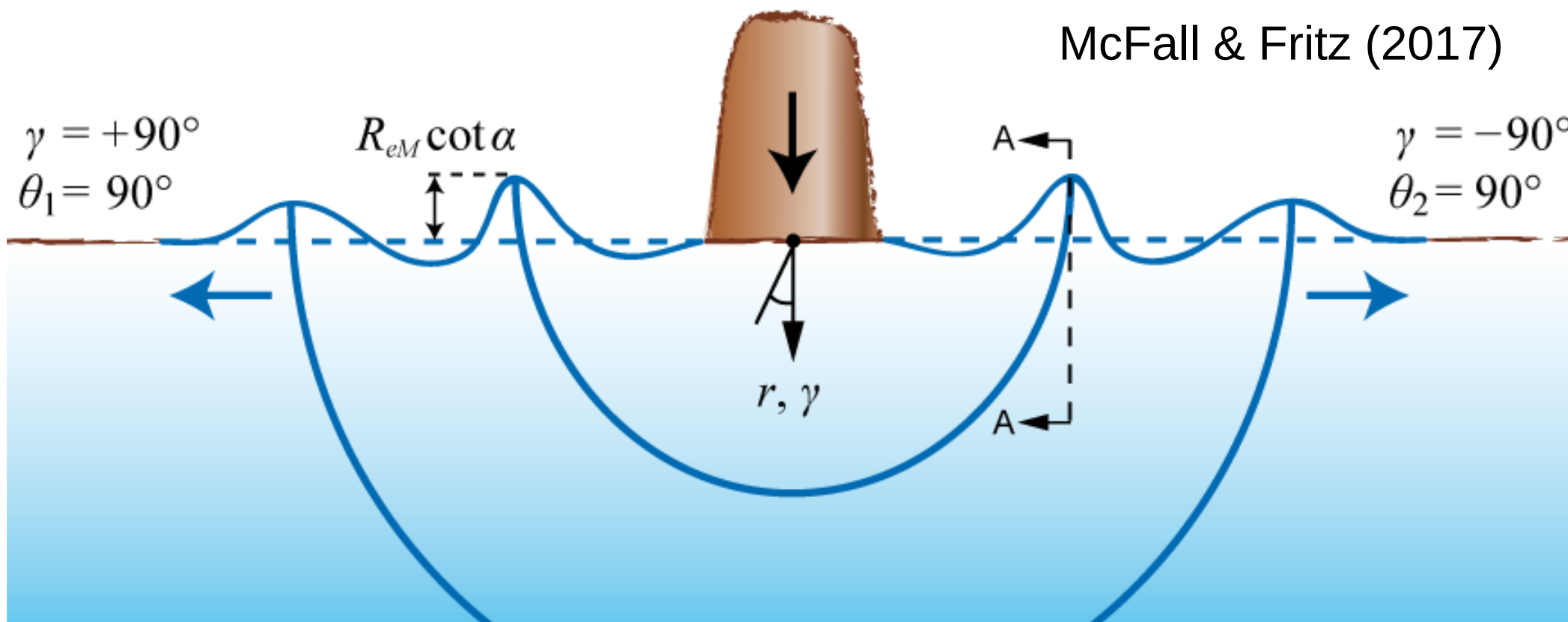
Edge waves - Experiment



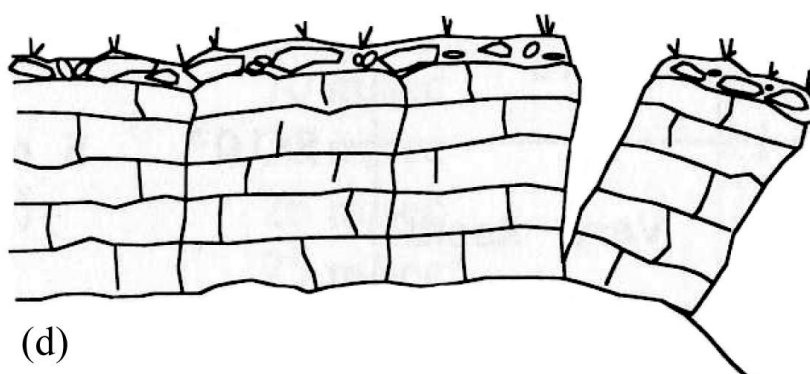
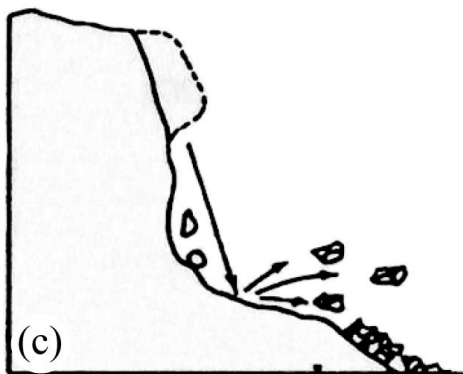
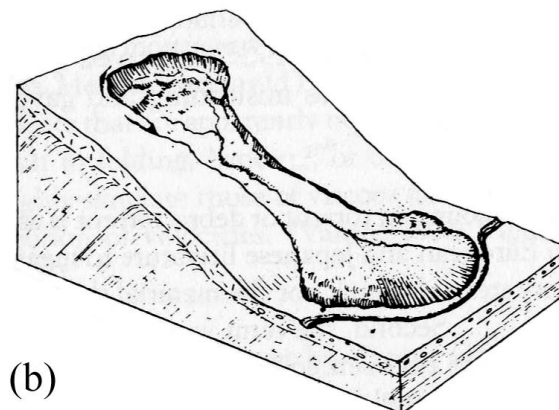
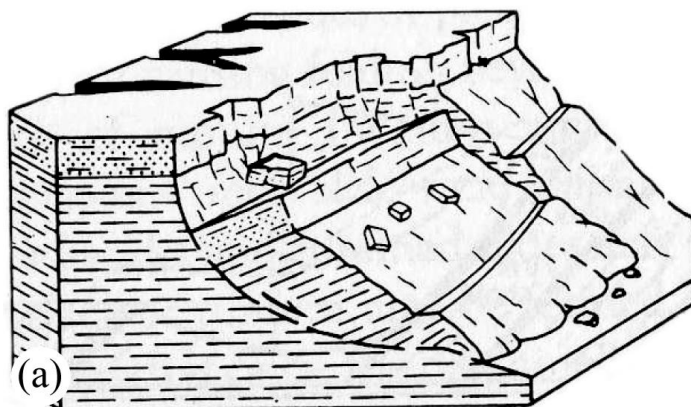
Edge waves

Heller & Spinncken (2015)

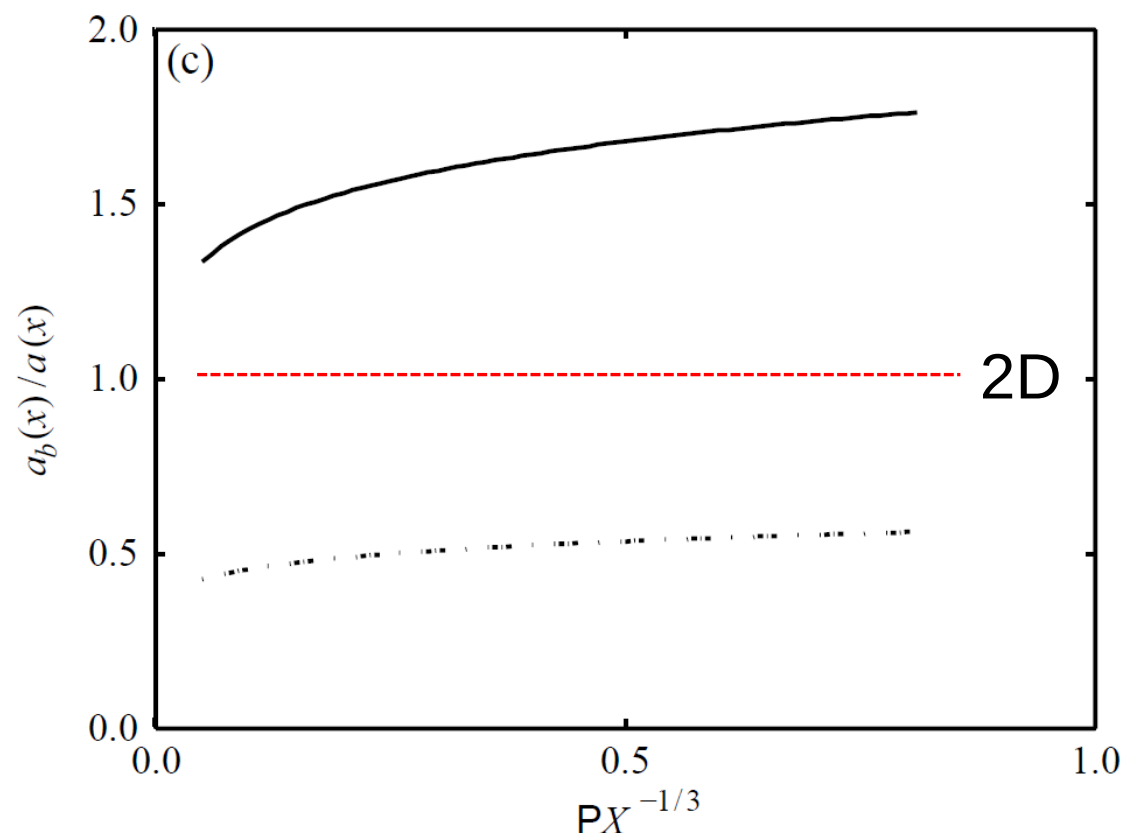
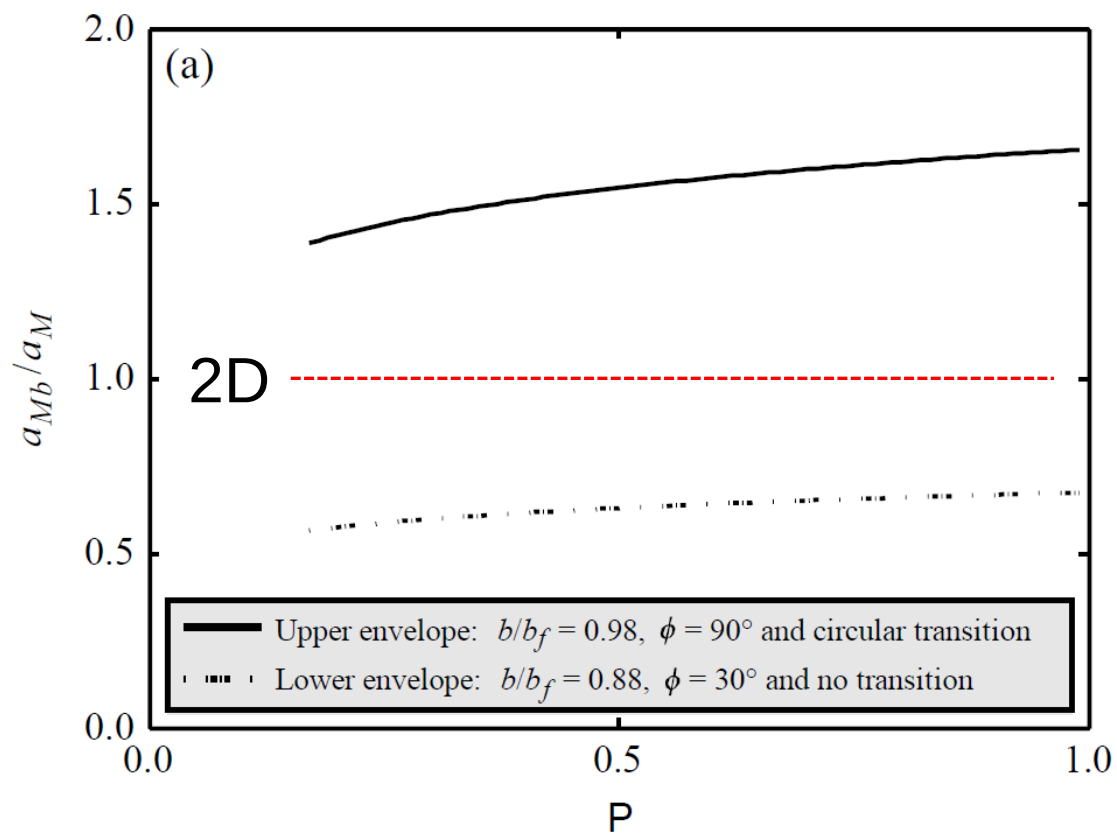
McFall & Fritz (2017)



Mass movement types

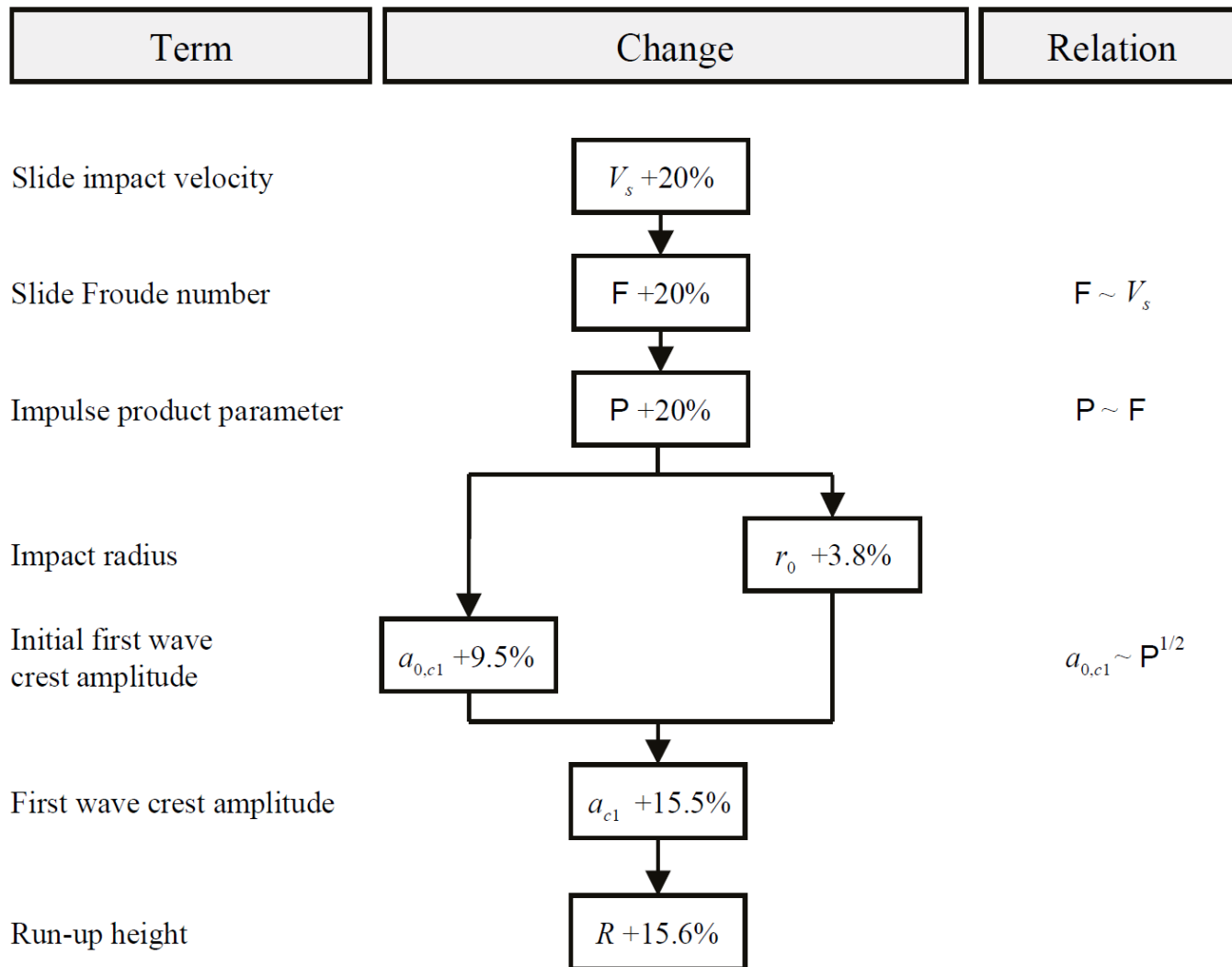
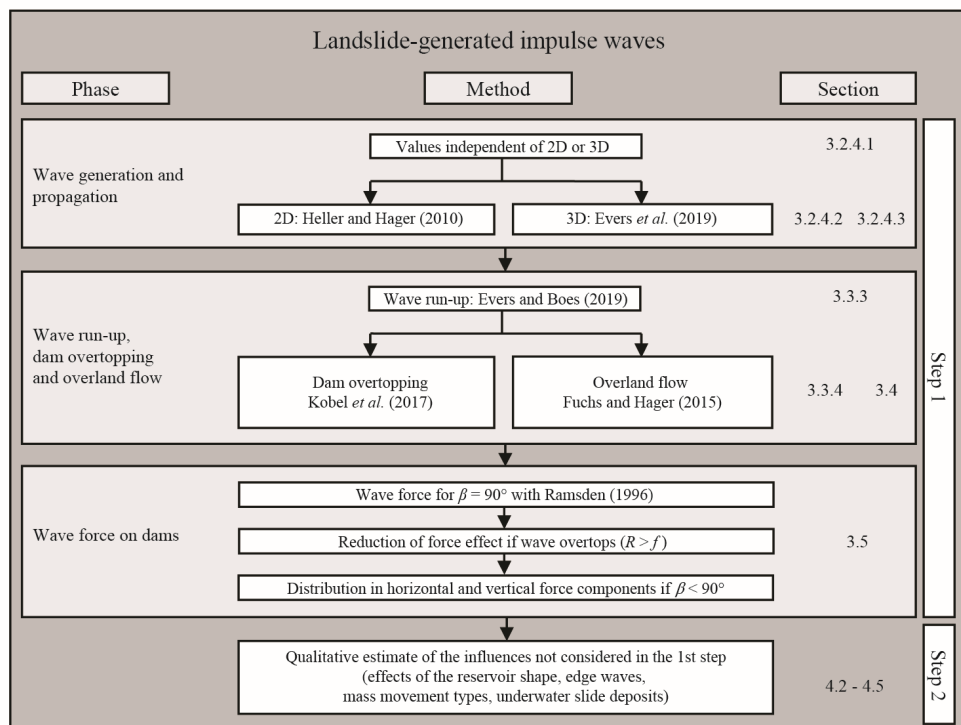


Mass movement types – Granular vs. Block

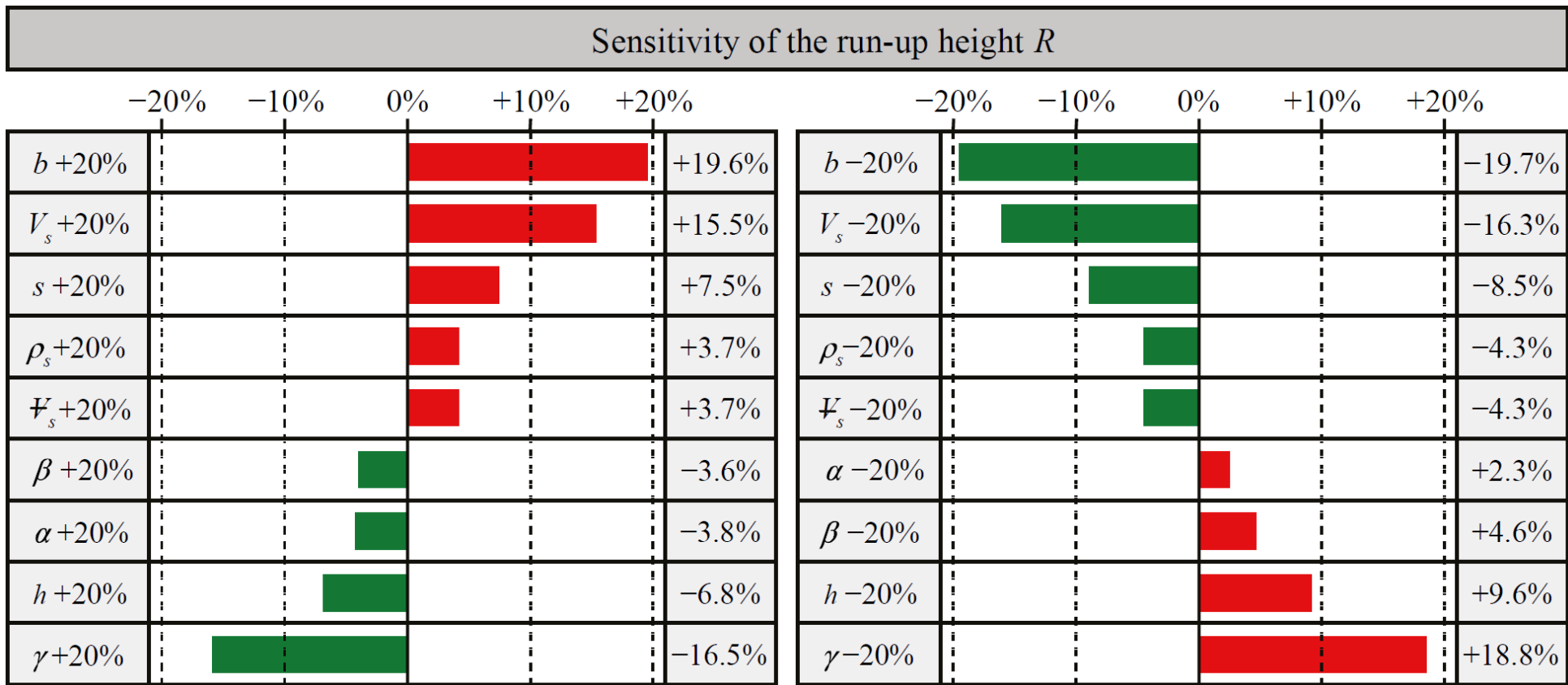


Heller & Spenneken (2013)

Sensitivity: V_s



Sensitivity: all parameters



Experimental scatter

Term	Symbol	Equation	Scatter
Maximum wave height (2D)	H_M	Eq. (3.13)	$\pm 30\%$
Streamwise distance of H_M (2D)	x_M	Eq. (3.14)	$\pm 50\%$
Wave period of H_M (2D)	T_M	Eq. (3.15)	$\pm 50\%$
Wave celerity (2D)	c	Eq. (3.17)	$\pm 15\%$
Wave height (2D)	H	Eq. (3.19)	$\pm 30\%$
Wave period (2D)	T	Eq. (3.20)	$\pm 100\%$
First wave crest amplitude (3D)	a_{c1}	Eq. (3.29)	+25%, -45%
First wave trough amplitude (3D)	a_{t1}	Eq. (3.30)	+40%, -25%
Second wave crest amplitude (3D)	a_{c2}	Eq. (3.31)	+50%, -60%
First wave crest celerity (3D)	c_{c1}	Eq. (3.32)	+10%, -15%
Second wave crest celerity (3D)	c_{c2}	Eq. (3.33)	+15%, -25%
First wave period (3D)	T_1	Eq. (3.34)	+15%, -10%
Run-up height	R	Eq. (3.36)	$\pm 20\%$
Overtopping volume per unit length dam crest (rigid dam)	V	Eq. (3.38)	$\pm 30\%$
Maximum overtopping flow depth (rigid dam)	d_0	Eq. (3.39)	$\pm 10\%$
Wave overtopping duration (rigid dam)	t_O	Eq. (3.40)	$\pm 10\%$



Conclusions «Step 2»

- Step 1 assumes simplified/idealized conditions
- Deviations from idealized conditions → Under-/Overestimation
- Safety allowance becomes necessary
- Additional topics: Ice cover, Underwater landslide deposits

